

OCTOBER 19, 2026



**Invitation to Bid –
Lisle Library District Nature Trail**

Addendum No. 02

Date Issued: October 19, 2026

Issued By: Bryon Krebs, Consertus

Addendum No.: 02

1.1 SUMMARY

A. The Invitation to Bid and Bidding Documents for the above referenced project are hereby modified and revised as follows.

B. The information contained within this addendum modifies, supplements or replaces information contained in the Bidding Documents and is hereby made a part of the Contract Documents.

C. Acknowledge receipt of this addendum in Section 1.19 of the Bid Form. FAILURE TO DO SO MAY SUBJECT THE BIDDER TO DISQUALIFICATION.

1.2 CLARIFICATIONS AND CORRECTIONS

Question/Request for Information #12: The documents refer to the bridge as both weathering steel and painted. Please verify the bridge finish. If it is to be painted, is it a 2-coat or 3-coat paint system?

Response/Clarification #12: The bridge is to be painted. Sheet S0.0 references Article 506.08 of the IDOT Standard Specifications for Road and Bridge Construction. Refer to that specification for information regarding the paint system.

Question/Request for Information #13: The drawings show the bridge with vertical pickets, a toe rail, and a pipe handrail, and also call out a smooth rub rail. Please verify the safety system: vertical pickets, toe rail, and rub rail, or vertical pickets, toe rail, and handrail.

Response/Clarification #13: The safety system shall be vertical pickets, toe rail, and rub rail, as specified in the notes on S0.0.

Question/Request for Information #14: Do the 1x6x6' dog-eared cedar boards and the 2x4x8' cedar rails need to be clear grade? Availability of clear grade material is a concern.

Response/Clarification #14: The wood fence specification is revised as follows. In Section 2.1, Item B, remove "clear" and replace it with "No. 2 / standard grade." In Section 2.1, Item C, remove "clear" and replace it with "No. 1 / select grade."

Question/Request for Information #15: Are there specifications for the H-piles, or is everything shown on the drawings?



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Response/Clarification #15: Pile specifications and sizes are shown on the drawings.

Question/Request for Information #16: Will the piles be bid as a lump sum or per linear foot?

Response/Clarification #16: Lump sum.

Question/Request for Information #17: A required tip elevation is shown, but no capacity is called out. Are the piles to be installed to a specific capacity?

Response/Clarification #17: The piles are to be installed to the required tip elevation. No capacities are given.

Question/Request for Information #18: The drawings reference a geotechnical report, but it was not included in the Bidding Documents. Please provide it.

Response/Clarification #18: The geotechnical report is attached.

Question/Request for Information #19: Please confirm the quantity of tortoise play features for the Lisle Library District Nature Trail.

Response/Clarification #19: Two tortoise play features are proposed. See revised sheet L1.0, attached.

Question/Request for Information #19: Can you provide cover for fixture pole receptacles?

Response/Clarification #19: Note will be added to Lighting Fixture Schedule, "Pole shall be supplied with lockable cover on in-base GFI receptacle".

1.3 DOCUMENT REVISIONS

The following Bidding Documents and drawings are revised by this Addendum:

1. Project Manual – Wood Fences specification
Summary of Changes:
 - i. Section 2.1, Item B: "clear" is replaced with "No. 2 / standard grade."
 - ii. Section 2.1, Item C: "clear" is replaced with "No. 1 / select grade."
2. Drawing L1.0 – Landscape Plan (revised sheet attached)
Summary of Changes:
 - i. Revised to show two tortoise play features. See Response #19.
3. Drawing S0.0 – General Notes (revised sheet attached)
Summary of Changes:
 - i. Revised to clarify that the steel is to be Grade 50. See Response #12.
4. Drawing S1.1 – General Details (revised sheet attached)
Summary of Changes:
 - i. Revised to include a rub rail in lieu of a handrail. See Response #13.
5. Geotechnical Report (attached)
Summary of Changes:
 - i. Added to the Bid Documents. See Response #18.

Please review accordingly and if necessary, make any changes or modifications to your bid documents for submittal and include this acknowledgement with your bid submittal. List this Addendum (Addendum

LISLE LIBRARY DISTRICT
NATURE TRAIL
INVITATION TO BID



OCTOBER 19, 2026

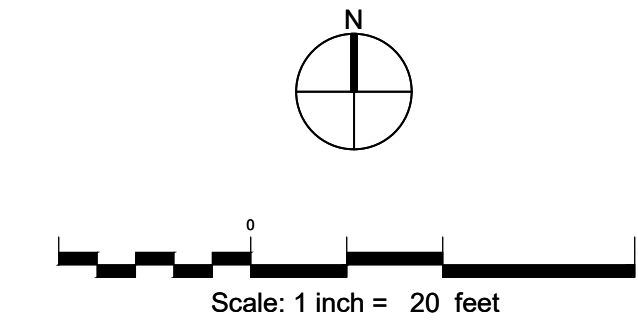
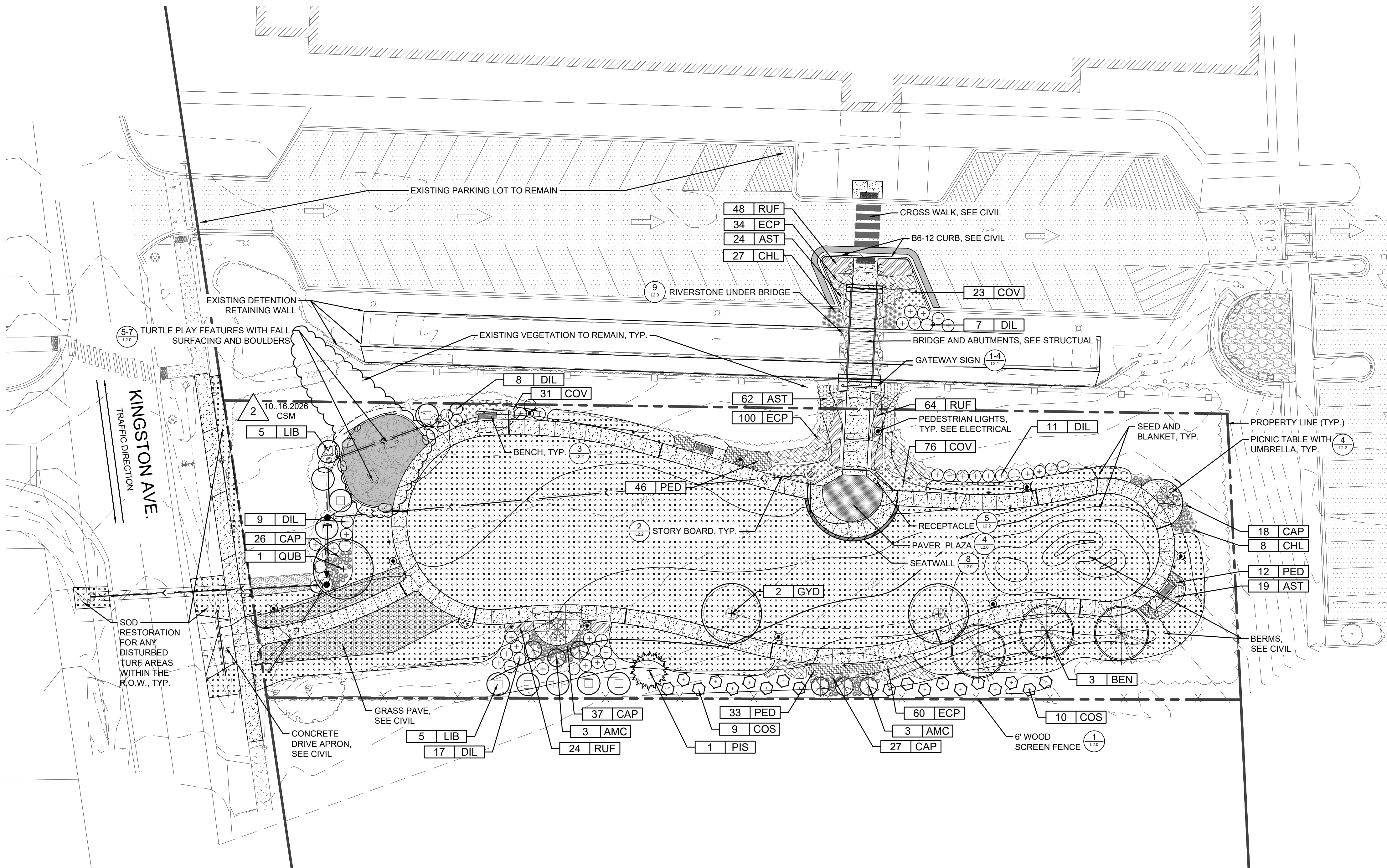
No. 02, dated October 19, 2026), along with Addendum No. 01 (dated October 14, 2026), in Section 1.19.A.7 of the Bid Form. Thank you in advance.

I Hereby Acknowledge Receipt of Addendum No. 02, Invitation to Bid (Lisle Library District Nature Trail)

Name and Title: *(Please Print)* _____

Signature: _____ **Date:** _____

Name of Company: _____



- PROPERTY LINE
- EXISTING BUILDING
- CONCRETE PAVEMENT AND BASE
- CONCRETE WALK AND BASE
- POURED IN PLACE PLAYGROUND SURFACE
- PAVERS
- GRASS PAVE
- CONCRETE CURB AND GUTTER
- RETAINING WALL
- WOOD FENCE
- LIGHT
- SIGN
- BOLLARD
- PICNIC TABLE WITH UMBRELLA
- BOULDERS
- SHADE TREE
- ORNAMENTAL TREE
- EVERGREEN TREE
- DECIDUOUS SHRUBS
- ORNAMENTAL GRASSES
- PERENNIAL / GROUNDCOVER AREAS
- SEED AND BLANKET
- MULCH, 3" DEPTH

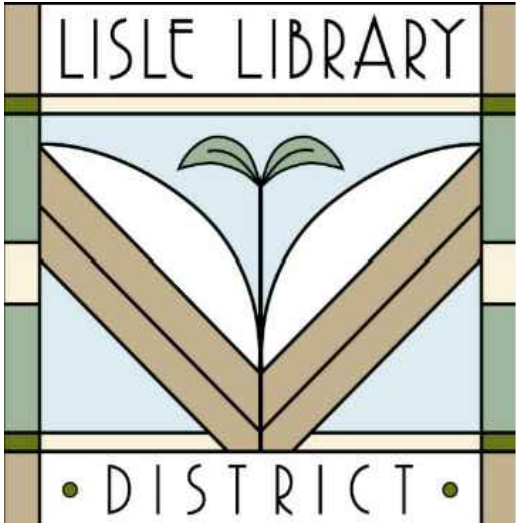
1 LANDSCAPE PLAN

SCALE: 1" = 20'

Plant Schedule

Lisle Library District Nature Trail

Qty.	Key	Botanical name	Common name	Size	Notes
SHADE TREES					
2	GYD	Gymnocladus dioicus 'Espresso'	Espresso Kentucky Coffeetree	2.5" B&B	Central leader
1	QUB	Quercus bicolor	Swamp White Oak	2.5" B&B	Central leader
EVERGREEN TREES					
1	PIS	Pinus strobus	White Pine	8' B&B	Central leader
ORNAMENTAL TREES					
6	AMC	Amelanchier canadensis 'Prince William'	Prince William Serviceberry	8' B&B	Multi-stem
3	BEN	Betula nigra 'Cully'	Cully Heritage River Birch	8' B&B	Clump (3 stem)
DECIDUOUS SHRUBS					
19	COS	Cornus sericea	Redosier Dogwood	30" B&B	6' o.c.
52	DIL	Diervilla 'G2X85411'	Kodiak Red Bush Honeysuckle	#5 Cont.	4' o.c.
10	LIB	Lindera benzoin	Spice Bush	48" B&B	10' o.c.
PERENNIALS					
105	AST	Asclepias tuberosa	Butterfly Milkweed	#1 Cont.	18" o.c.
130	COV	Coreopsis verticillata 'Moonbeam'	Moonbeam Coreopsis	#1 Cont.	18" o.c.
194	ECP	Echinacea purpurea 'Magnus'	Magnus Purple Coneflower	#1 Cont.	18" o.c.
91	PED	Penstemon digitalis 'Husker Red'	Husker Red Beardtongue	#1 Cont.	18" o.c.
136	RUF	Rudbeckia fulgida var. sullivantii	Showy Black-Eye Susan	#1 Cont.	18" o.c.
ORNAMENTAL GRASSES					
108	CAP	Carex pensylvanica	Common Oak Sedge	#1 Cont.	18" o.c.
35	CHL	Chasmanthium latifolium	Northern Sea Oats	#1 Cont.	18" o.c.



LISLE LIBRARY DISTRICT NATURE TRAIL

4931 Kingston Avenue
Lisle, IL 60532

Client

LISLE LIBRARY DISTRICT

777 Front Street
Lisle, IL 60532

Project Team



Revisions

Date	Issue
10.02.2026	IFB/IFP
10.16.2026	ADDENDUM 2

Stamp

Title

LANDSCAPE PLAN

Sheet No.

L1.0

- Notes:
1. All exposed substructure concrete shall be coated with "Concrete Sealer" in accordance with Section 587 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.
2. Bridge Deck shall be coated with "Protective Coat" in accordance with Section 1023 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.
3. Concrete work shall conform with Section 503 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.
4. Reinforcement Bars shall conform with Section 508 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.
5. Flowable fill shall conform with Section 593 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.
6. Preformed Expansion Joint Filler shall be according to Article 1051.09 of the IDOT 2022 Standard Specifications for Road and Bridge Construction.

August 31, 2026

L-101,160



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Geotechnical & Environmental Engineering



Construction Materials Engineering & Testing



Laboratory Testing of Soils, Concrete & Asphalt



Geo-Environmental Drilling & Sampling

Report of Soils Exploration

Lisle Library Improvements

Gateway Sign, Pedestrian Bridge & Nature Trail

777 Front Street

Lisle, Illinois

Prepared For:

Lisle Library District

777 Front Street

Lisle, IL 60532

GEOTECHNICAL GROUP





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August 31, 2026

Ms. Tatiana Weinstein
Lisle Library District
777 Front Street
Lisle, IL 60532

Re: L-101,160
Lisle Library Improvements
Gateway Sign, Pedestrian Bridge & Nature Trail
777 Front Street
Lisle, Illinois

Dear Ms. Weinstein:

This report presents results of a soils exploration performed for the proposed Lisle Library Improvements in Lisle, Illinois. These geotechnical services are being provided in accordance with TSC Proposal No. 78,315 dated June 22, 2026 and the attached General Conditions, incorporated herein by reference.

The project area consists of the parking lot outside of the upper (south) entrance of the Lisle Library and the adjoining grassy parcel to the south. An existing detention basin surrounded by a retaining wall structure is present between these two properties.

Current plans call for improvements to Lisle Library. These improvements are to include a gateway sign on the north end of the site and a proposed pedestrian bridge to span across the existing detention basin. Finally, a nature trail is planned around the southern property.

Based on information provided by Mr. Dan Carroll, P.E. of Terra Engineering Ltd., the pedestrian bridge will consist of a single-span prefabricated truss bridge structure. The new bridge will have a length of approximately 32'-3" and an out-to-out width of 9'-0". It is assumed that the bridge abutments are likely to be supported on either pile foundations or shallow foundations.

It is understood that the Gateway sign structure is likely to be between 12 and 14 feet high. Additional information about the Gateway sign and nature trail was not available at the time of this report.

Field Investigation and Laboratory Testing

Borings 1 - 6 were drilled as part of this subsurface exploration. A Boring Location Plan is enclosed showing the drilling layout. Ground surface elevations at the boring locations are also indicated, being acquired using a Trimble R12 GNSS receiver and rounded to the nearest 0.5 foot.

Borings 1 and 2 were drilled for the proposed gateway sign and pedestrian bridge; they were extended 30 feet below existing grade. Borings 3 - 6 were drilled for the proposed nature trail and were made 10 feet deep. The borings were drilled and samples tested according to currently recommended American

Society for Testing and Materials (ASTM) specifications. Soil sampling was performed at 2½-foot intervals with samples taken in conjunction with the Standard Penetration Test (SPT), for which driving resistance to a 2" split-spoon sampler (N-value in blows per foot) provides an indication of the relative density of granular materials and consistency of cohesive soils. Water level readings were taken during and following completion of drilling operations, with the boreholes immediately backfilled for safety reasons.

Soil samples were examined in the laboratory to verify field descriptions and to classify them in accordance with the Unified Soil Classification System (USCS). Laboratory testing included moisture content determinations for all cohesive and intermediate (silt and loamy) soil types along with dry unit weight determinations on cohesive fill. An estimate of unconfined compressive strength was obtained for cohesive soils using a calibrated pocket penetrometer (Qp), with actual measurements of unconfined strength (Qu) performed on representative samples of inorganic native clay soils.

For classification purposes and to verify descriptions, one (1) grain-size analysis (sieve plus hydrometer) and one (1) Atterberg Limit test were performed on a combined representative soil sample. The results of these tests are summarized on the Soil Test Data Sheets which are included in the Appendix.

In addition, grain size analyses were performed on two (2) samples taken at the approximate depth of the proposed infiltration zone at approximately 5 feet below existing grade in Boring 2. Results of these tests as well as the Unified and USDA classifications are summarized in a separate table ("Soil Test Data") attached. Based on the USDA classification, the design infiltration rate of these granular materials was estimated using Design Infiltration Rates for USDA Soil Textures (University of Wisconsin, Madison, 2006).

Reference is made to the attached boring logs indicating subsurface stratigraphy and soil descriptions, results of field and laboratory tests, as well as water level observations. Definitions of descriptive terminology are also included. While strata changes are shown as a definite line on the boring logs, the actual transition between soil layers may be more gradual. Fluctuations in groundwater levels may also occur due to variations in precipitation (short-term and seasonal), i.e. water levels at a future date may be higher or lower than those recorded at the time of drilling.

Discussion of Test Data

Boring 1 was drilled on the existing library parking lot and revealed 5 inches bituminous concrete at the surface overlying about 8 inches of crushed stone. These measurements were estimated from the disturbed sides of the auger boreholes and should be considered approximate; pavement cores may be taken if more accurate measurements or descriptions of the pavement are required.

Borings 2 - 6 were drilled on the southern grassy lot. Surficial topsoil was 10 to 18 inches thick at the boring locations. The thicker clayey topsoil deposits exhibited water contents of 29 to 33 percent.

Stiff to very stiff native silty clay soils of apparent medium to high plasticity were found below the surficial topsoil, in Borings 2, 3 and 5, extending to depths of about 3 feet below existing grade. These

borderline CL/CH materials (Unified classification) were typified by moderate unconfined compressive strengths ranging from 2.0 to 3.0 tons per square foot (tsf), at high water contents of 25 to 28 percent.

Medium stiff to hard native silty clay and sandy clay soils of low to moderate plasticity otherwise predominated and extended to the bottom of the borings. These deeper cohesive materials exhibited unconfined compressive strengths typically ranging from 1.25 to 4.5+ tsf, while being as low as 0.75 tsf in the upper 8 feet of Boring 1. Water contents were in the range of 13 to 23 percent. A layer of medium dense silty sand was interbedded in the greater cohesive soil mass in Boring 2. These intermediate materials exhibited SPT N-values of 13 blows per foot (bpf).

Free water was initially encountered at about 1 and 20 feet below existing grade in Borings 1 and 2, respectively. The free water encountered in Boring 1 likely represents a perched condition in the granular base course pavement section. The water levels in Borings 1 and 2 were about 1 and 23 feet deep upon completion of drilling operations. The remaining shallower borings were all "dry" both during and following completion of drilling operations.

Analysis and Recommendations

Proposed Gateway Sign and Bridge Structure (Borings 1 & 2)

It is understood that a 12' to 14' tall gateway sign is planned near Boring 1 in the Lisle Library parking lot. The specific location, diameter and depths of the proposed sign foundations were not known at the time of this report.

Additionally, a pedestrian bridge is proposed to span across the existing detention basin. Based on information provided by Mr. Dan Carroll, P.E. of Terra Engineering Ltd., the pedestrian bridge is to be a single-span prefabricated truss bridge structure. The new bridge will have a length of approximately 32'-3" and an out-to-out width of 9'-0". It is assumed that the bridge abutments are likely to be supported on pile foundations.

Soil Parameters for L-Pile Design

Regarding lateral load pile analysis, the following table summarizes the recommended soil parameters that are recommended in connection with the p-y method (L-Pile computer program) in connection with the gateway sign structure. The upper zone of soil, extending to at least the depth of frost protection (4.0 feet), is typically neglected for lateral resistance to the foundation. Effective stress analysis would require utilizing moist unit weight values above groundwater elevation in the borings and buoyant unit weights below. It is estimated that the actual phreatic surface is located at about Elevation 707 at the borings.

Soil Parameters for Analysis of Piles under Lateral Loads

Boring-1				
Elevation	Soil Type (p-y curve model)	Effective Unit Weight (psf)	Su (psf)	Phi ϕ' (deg.)
715.0 - 716.7	Stiff Clay w/o Free Water	133	4000	0
710.0 - 715.0	Stiff Clay w/o Free Water	125	750	0
707.5 - 710.0	Stiff Clay w/o Free Water	133	4500	0
705.0 - 707.5	Stiff Clay w/o Free Water	71*	4000	0
700.0 - 705.0	Stiff Clay w/o Free Water	66*	1500	0
688.0 - 700.0	Stiff Clay w/o Free Water	68*	2750	0

Boring-2				
Elevation	Soil Type (p-y curve model)	Effective Unit Weight (psf)	Su (psf)	Phi ϕ' (deg.)
719.5 - 721.7	Stiff Clay w/o Free Water	130	3000	0
707.0 - 719.5	Stiff Clay w/o Free Water	133	3800	0
702.0 - 707.0	Stiff Clay w/o Free Water	68*	2000	0
699.5 - 702.0	Sand (Reese)	58*	0	33
692.5 - 699.5	Stiff Clay w/o Free Water	66*	1500	0

* Effective unit weight of soil below the normal water level of Elevation 707.0.

Su - Undrained Shear Strength in pounds per square foot (psf).

Phi ϕ' - Angle of Internal Friction (degrees).

Pile Foundations

Consideration may be given to supporting the bridge abutments on steel H-piles. Two (2) pile sections have been evaluated: Steel HP 10x42 and 12x63. The following table provides the maximum Nominal Required Bearing (R_N) and maximum Factor Resistance Available (R_F) for each of the different piles.

Pile Designation	Ultimate Required Bearing (kips)	Allowable Resistance Available (kips) *
Steel HP 10x42	335	112
Steel HP 12x63	497	166

- * Allowable Resistance Available computed using a factor of safety of 3.0; no reduction taken for scour, downdrag or liquefaction.

Since abutment locations and depths were unknown at the time this report was written, it was assumed that the bottom of the pile caps would extend a minimum of 4 feet below existing grade, i.e. frost depth. The estimated pile lengths/elevations for the bridge abutments are summarized in the following table using the Modified IDOT Static Method (Microsoft Excel program). Two (2) typical pile sections have been evaluated for four (4) general Nominal Required Bearing (R_N) values. They have been prepared in connection with Design Guide 3.10.1, LRFD Geotechnical Pile Design Procedure and AGMU Memo 10.2 (Geotechnical Pile Design). The estimated pile lengths include 1.0 foot of embedment into the abutment pile cap. No reduction was taken for downdrag, scour or liquefaction in computing the Factored Resistance Available.

Boring 1 North Abutment		Nominal Required Bearing - R_N (kips)			
		36	55	73	91
Pile Designation Bottom Pile Cap Elev. = 716.0		Factored Resistance Available - R_F (kips) #			
		20	30	40	50
HP 10x42	Length (ft)*	10	12	17	22
	Elevation**	707	705	700	695
HP 12x53	Length (ft)*	9 ^{##}	10	13	17
	Elevation**	708	707	704	700

Boring 2 South Abutment		Nominal Required Bearing - R_N (kips)			
		36	55	73	91
Pile Designation Bottom Pile Cap Elev. = 715.0		Factored Resistance Available - R_F (kips) #			
		20	30	40	50
HP 10x42	Length (ft)*	6 ^{##}	8 ^{##}	11	18
	Elevation**	710	708	705	698
HP 12x53	Length (ft)*	6 ^{##}	7 ^{##}	9 ^{##}	10
	Elevation**	710	709	707	706

NA Nominal Required Bearing exceeds the maximum nominal required bearing for the pile.

* The estimated pile length includes 1.0 foot of embedment into the abutment pile cap, being rounded to the nearest foot.

** The estimated bottom of the pile elevation to achieve the Factored Resistance Available, being rounded to the nearest foot.

- # Factored Resistance Available was computed using a geotechnical resistance factor of 0.55. Due to the presence of cobbles within the soil stratigraphy, it is recommended that the piles be provided with metal pile shoes (pile points). The above estimated pile lengths are being provided for contract estimates. They were estimated using the Modified IDOT Static Method and the soils revealed by the borings. It should be noted that the length of piles not driven to rock are more difficult to predict.
- ## Although the calculated factored resistance in the table above indicates that the design load requirements can be satisfied at relatively shallow pile embedment depths, pile installation should be governed by the minimum penetration requirements specified in the applicable Standard Specifications for Road and Bridge Construction. Specifically, piles should be driven to a minimum penetration of 10 feet below the bottom of the footing, regardless of the depth at which the required factored resistance is achieved.

The actual pile lengths should be determined during installation based on resistance to driving criteria. It is recommended that at least one test pile be driven at each substructure prior to ordering piles for production driving. The test piles are normally driven to 110 percent of the Allowable Required Bearing shown on the plans. It is also recommended that piles that have been driven to their full furnished length and have not reached but are within 85% of the full nominal required bearing should be left for a minimum of 24 hours to allow soil set-up to occur. This allows excess pore water pressures to dissipate and reconsolidation of the soil around the pile to occur, over time resulting in an increase in pile capacity.

The driving equipment should be selected so that the piles can be driven to the required capacity (Ultimate Required Bearing) at an adequate final penetration resistance and without inducing pile stresses that exceed allowable values. A wave equation analysis may be performed to further evaluate pile driveability which is to ultimately be confirmed during the driving of test piles. Dynamic pile testing may also be performed during test pile driving operations to more accurately assess the capacity being achieved by the piles (nominal driven bearing) and to establish the driving criteria.

Spread Footings (Boring 1 - North Side)

The feasibility of supporting the new bridge structure on spread footing foundations was evaluated. For discussion purposes, it is assumed that the bridge abutment will bear about 4 feet below finished grade (frost depth), corresponding to about Elevation 715.16. Slope protection and erosion control measures should be implemented to mitigate the potential for erosion and subsequent undermining of the new bridge foundations.

Subsurface conditions encountered in Boring 1 revealed the presence of medium stiff silty clay soils at the anticipated bridge bearing elevation. These soils are marginally suitable for bridge foundation support. The clay soils are capable of supporting of a nominal bearing resistance of 4000 pounds per square foot (psf). The factored bearing resistance is calculated using a geotechnical resistance factor of 0.5 and the nominal bearing resistance. Therefore, a factored bearing resistance of 2000 psf can be used for shallow abutment foundations design in connection with 4000 psf nominal bearing resistance soils. However, once the bridge abutments depth and size have been determined the bearing resistance values should be recalculated to verify the correct interpretation of recommendations contained herein and to modify the findings accordingly.

If the bridge foundations are extended down to the hard silty clay soils encountered at approximate Elevation 710.0 a high bearing capacity can be used for design. The hard clay soils are capable of

supporting of a nominal bearing resistance of 15,000 pounds per square foot (psf). The factored bearing resistance is calculated using a geotechnical resistance factor of 0.5 and the nominal bearing resistance. Therefore, a factored bearing resistance of 7500 psf can be used for shallow abutment foundations design in connection with 15,000 psf nominal bearing resistance soils.

Please note that the bridge abutment foundations should always be provided with a minimum of 4.0 feet of cover for frost protection. Also, once the initial foundation excavation has been made, a qualified soils technician or engineer should inspect the foundation soils to confirm that suitable bearing soils have been reached. All loose and disturbed soil should be removed from the excavation.

Spread Footings (Boring 2 - South Side)

The feasibility of supporting the new bridge structure on spread footing foundations was evaluated. For discussion purposes, it is assumed that the bridge abutments will bear about 4 feet below finished grade (frost depth), corresponding to about Elevation 713.78 for the south abutment. Slope protection and erosion control measures should be implemented to mitigate the potential for erosion and subsequent undermining of the new bridge foundations.

Subsurface conditions encountered in Boring 2 revealed the presence of very stiff silty clay soils at the anticipated bridge bearing elevation. The very stiff silty clay soils are capable of supporting of a nominal bearing resistance of 15,000 pounds per square foot (psf). The factored bearing resistance is calculated using a geotechnical resistance factor of 0.5 and the nominal bearing resistance. Therefore, a factored bearing resistance of 7500 psf can be used for shallow abutment foundations design in connection with 15,000 psf nominal bearing resistance soils.

Please note that the bridge abutment foundations should always be provided with a minimum of 4.0 feet of cover for frost protection. Also, once the initial foundation excavation has been made, a qualified soils technician or engineer should inspect the foundation soils to confirm that suitable bearing soils have been reached. All loose and disturbed soil should be removed from the excavation.

Infiltration (Boring 2)

It is understood that proposed infiltration is being considered near the south end of the proposed bridge structure at a depth of approximately 5 feet. Sieve analyses were run on two (2) representative samples at the approximate proposed infiltration depth. The samples revealed between 20 and 52 percent sand and gravel sized particles, with 48 to 80 percent fines passing the #200 sieve (silt and clay). These clay are all classified as Silty Clay Loam and Clay Loam using the USDA classification system. Based on the Design Infiltration Rates for USDA Soil Textures (University of Wisconsin, Madison, 2006), infiltration rates of between 0.03 and 0.19 in/hr are indicated for Very Gravelly Clay Loam and Silty Clay Loam soil types, respectively. These results can also be seen on the Soil Test Data sheet attached.

Pavement Design and Construction (Borings 3 - 6)

It is understood that a nature trail is planned for the southern grassy lot. Pavement subgrade preparation, for the proposed nature trail, should include stripping of surficial topsoil/root zone materials and proof-rolling. The silty clay soils were found below the topsoil layer at all the boring locations. The

clay soils are generally considered suitable for pavement subgrade support. However, it should be anticipated that the clay soils in all areas will have to be reduced in moisture content and recompacted prior to paving. Compacted to 95 percent of Modified Proctor density is recommended to create a stable base for proof-rolling and paving.

The proof-roll should be performed using a loaded dump truck or other piece of heavy rubber-tired construction equipment, in order to check for the presence of unsuitable soil types. In this regard, the exposed clay subgrade at some of the core locations will have to be reduced in water content and/or recompacted in order to provide a stable subgrade prior to paving. If paving construction is performed when drying of surficial soils cannot be accomplished, lime or cement modification or removal of unstable subgrade and replacement with 1 to 2 feet of coarse granular materials may be required. Granular materials should consist of non-expansive crushed concrete, crushed stone or crushed gravel between ¼ to 3 inches in size and containing no fines; IDOT gradations CA-1 and CA-7 meet these criteria.

It is recommended that a nominal California Bearing Ratio (CBR) value of 3.0 be used in the design of asphalt pavements. A modulus of subgrade reaction of 150 pci may be used for rigid pavement design. Use of these values assumes that any soft or unstable areas will be remediated, i.e. subgrade stabilized until passing a proof-roll.

Base course materials should conform be compacted to 95 percent Modified Proctor density or 100 percent of the Standard Proctor (ASTM D 698) maximum density value. Bituminous materials (if being utilized) should conform to an approved IDOT Superpave mix design, with N30 or N50 typical for light-duty parking lots and N50 or N70 for heavy-duty pavements. Standard Specifications for Road and Bridge Construction, Sections 406 and 1032 should also be referenced. They should be compacted to between 93 and 97 percent of their theoretical maximum density, as determined by the supplier.

Groundwater Management

Groundwater was encountered at depths of 1 and 20 feet while drilling Borings 1 and 2, respectively. The groundwater encountered in Boring 1 likely represents a perched condition in the pavement section. Therefore, the accumulation of run-off water or seepage at the base of excavations should be expected to occur during foundation construction and site work. The Contractor should be prepared to remove these accumulations.

Closure

It is recommended that full-time Construction Materials Testing and Engineering (CME) services be provided by Testing Service Corporation personnel during foundation construction, so that the soils at foundation levels can be observed and tested. In addition, stripping and undercutting, fill placement and compaction as well as slab-on-grade and pavement construction should be observed and tested for compliance with the recommended procedures and specifications.

This soils report has been prepared without the benefit of sign and bridge structure foundations or related information. It is therefore suggested that Testing Service Corporation review these plans when

they are available, to check the accuracy of this report as it may be affected, to verify the correct interpretation of recommendations contained herein and to modify the findings accordingly.

The analysis and recommendations submitted in this report are based upon the data obtained from the six (6) soil borings performed at the locations indicated on the Boring Location Plan. This report does not reflect any variations which may occur, the nature and extent of which may not become evident until during the course of construction. If variations are then identified, recommendations contained in this report should be re-evaluated after performing on-site observations.

Please call if there are any questions in regard to this matter or if we may be of further service.

Respectfully submitted,

TESTING SERVICE CORPORATION

A handwritten signature in blue ink, appearing to read "Sam Patrick".

Samuel J. Patrick, P.E.
Project Engineer
Licensed Professional Engineer
Illinois No. 062-073810

A handwritten signature in blue ink, appearing to read "Tim Peceniak".

Timothy R. Peceniak, P.E.
Geotechnical Engineer

SJP:TRP:sjp



TESTING SERVICE CORPORATION

1. PARTIES AND SCOPE OF WORK: If Client is ordering the services on behalf of another, Client represents and warrants that Client is the duly authorized agent of said party for the purpose of ordering and directing said services, and in such case the term "Client" shall also include the principal for whom the services are being performed. Prices quoted and charged by TSC for its services are predicated on the conditions and the allocations of risks and obligations expressed in these General Conditions. Unless otherwise stated in writing, Client assumes sole responsibility for determining whether the quantity and the nature of the services ordered by Client are adequate and sufficient for Client's intended purpose. Unless otherwise expressly assumed in writing, TSC's services are provided exclusively for client. TSC shall have no duty or obligation other than those duties and obligations expressly set forth in this Agreement. TSC shall have no duty to any third party. Client shall communicate these General Conditions to each and every party to whom the Client transmits any report prepared by TSC. Ordering services from TSC shall constitute acceptance of TSC's proposal and these General Conditions.

2. SCHEDULING OF SERVICES: The services set forth in this Agreement will be accomplished in a timely and workmanlike manner. If TSC is required to delay any part of its services to accommodate the requests or requirements of Client, regulatory agencies, or third parties, or due to any cause beyond its reasonable control, Client agrees to pay such additional charges, if any, as may be applicable.

3. ACCESS TO SITE: TSC shall take reasonable measures and precautions to minimize damage to the site and any improvements located thereon as a result of its services or the use of its equipment; however, TSC has not included in its fee the cost of restoration of damage which may occur. If Client desires or requires TSC to restore the site to its former condition, TSC will, upon written request, perform such additional work as is necessary to do so and Client agrees to pay to TSC the cost thereof plus TSC's normal markup for overhead and profit.

4. CLIENT'S DUTY TO NOTIFY ENGINEER: Client represents and warrants that Client has advised TSC of any known or suspected hazardous materials, utility lines and underground structures at any site at which TSC is to perform services under this Agreement. Unless otherwise agreed in writing, TSC's responsibility with respect to underground utility locations is to contact the Illinois Joint Utility Locating Information for Excavators for the location of public, but not private, utilities.

5. DISCOVERY OF POLLUTANTS: TSC's services shall not include investigation for hazardous materials as defined by the Resource Conservation Recovery Act, 42 U.S.C. § 6901, et seq., as amended ("RCRA") or by any state or Federal statute or regulation. In the event that hazardous materials are discovered and identified by TSC, TSC's sole duty shall be to notify Client.

6. MONITORING: If this Agreement includes testing construction materials or observing any aspect of construction of improvements, Client's construction personnel will verify that the pad is properly located and sized to meet Client's projected building loads. Client shall cause all tests and inspections of the site, materials and work to be timely and properly performed in accordance with the plans, specifications, contract documents, and TSC's recommendations. No claims for loss, damage or injury shall be brought against TSC unless all tests and inspections have been so performed and unless TSC's recommendations have been followed.

TSC's services shall not include determining or implementing the means, methods, techniques or procedures of work done by the contractor(s) being monitored or whose work is being tested. TSC's services shall not include the authority to accept or reject work or to in any manner supervise the work of any contractor. TSC's services or failure to

perform same shall not in any way operate or excuse any contractor from the performance of its work in accordance with its contract. "Contractor" as used herein shall include subcontractors, suppliers, architects, engineers and construction managers.

Information obtained from borings, observations and analyses of sample materials shall be reported in formats considered appropriate by TSC unless directed otherwise by Client. Such information is considered evidence, but any inference or conclusion based thereon is, necessarily, an opinion also based on engineering judgment and shall not be construed as a representation of fact. Subsurface conditions may not be uniform throughout an entire site and ground water levels may fluctuate due to climatic and other variations. Construction materials may vary from the samples taken. Unless otherwise agreed in writing, the procedures employed by TSC are not designed to detect intentional concealment or misrepresentation of facts by others.

7. DOCUMENTS AND SAMPLES: Client is granted an exclusive license to use findings and reports prepared and issued by TSC and any sub-consultants pursuant to this Agreement for the purpose set forth in TSC's proposal provided that TSC has received payment in full for its services. TSC and, if applicable, its sub-consultant, retain all copyright and ownership interests in the reports, boring logs, maps, field data, field notes, laboratory test data and similar documents, and the ownership and freedom to use all data generated by it for any purpose. Unless otherwise agreed in writing, test specimens or samples will be disposed immediately upon completion of the test. All drilling samples or specimens will be disposed sixty (60) days after submission of TSC's report.

8. TERMINATION: TSC's obligation to provide services may be terminated by either party upon (7) seven days prior written notice. In the event of termination of TSC's services, TSC shall be compensated by Client for all services performed up to and including the termination date, including reimbursable expenses. The terms and conditions of these General Conditions shall survive the termination of TSC's obligation to provide services.

9. PAYMENT: Client shall be invoiced periodically for services performed. Client agrees to pay each invoice within thirty (30) days of its receipt. Client further agrees to pay interest on all amounts invoiced and not paid or objected to in writing for valid cause within sixty (60) days at the rate of twelve (12%) per annum (or the maximum interest rate permitted by applicable law, whichever is the lesser) until paid and TSC's costs of collection of such accounts, including court costs and reasonable attorney's fees.

10. WARRANTY: TSC's professional services will be performed, its findings obtained and its reports prepared in accordance with these General Conditions and with generally accepted principles and practices. In performing its professional services, TSC will use that degree of care and skill ordinarily exercised under similar circumstances by members of its profession. In performing physical work in pursuit of its professional services, TSC will use that degree of care and skill ordinarily used under similar circumstances. This warranty is in lieu of all other warranties or representations, either express or implied. Statements made in TSC reports are opinions based upon engineering judgment and are not to be construed as representations of fact.

Should TSC or any of its employees be found to have been negligent in performing professional services or to have made and breached any express or implied warranty, representation or contract, Client, all parties claiming through Client and all parties claiming to have in any way relied upon TSC's services or work agree that the maximum aggregate amount of damages for which TSC, its officers, employees and agents shall be liable is limited to \$50,000 or the total amount of the fee paid to TSC for its services performed with respect to the project, whichever amount is greater.

In the event Client is unwilling or unable to limit the damages for which TSC may be liable in accordance with the provisions set forth in the preceding paragraph, upon written request of Client received within five days of Client's acceptance of TSC's proposal together with payment of an additional fee in the amount of 5% of TSC's estimated cost for its services (to be adjusted to 5% of the amount actually billed by TSC for its services on the project at time of completion), the limit on damages shall be increased to \$500,000 or the amount of TSC's fee, whichever is the greater. This charge is not to be construed as being a charge for insurance of any type, but is increased consideration for the exposure to an award of greater damages.

11. INDEMNITY: Subject to the provisions set forth herein, TSC and Client hereby agree to indemnify and hold harmless each other and their respective shareholders, directors, officers, partners, employees, agents, subsidiaries and division (and each of their heirs, successors, and assigns) from any and all claims, demands, liabilities, suits, causes of action, judgments, costs and expenses, including reasonable attorneys' fees, arising, or allegedly arising, from personal injury, including death, property damage, including loss of use thereof, due in any manner to the negligence of either of them or their agents or employees or independent contractors. In the event both TSC and Client are found to be negligent or at fault, then any liability shall be apportioned between them pursuant to their pro rata share of negligence or fault. TSC and Client further agree that their liability to any third party shall, to the extent permitted by law, be several and not joint. The liability of TSC under this provision shall not exceed the policy limits of insurance carried by TSC. Neither TSC nor Client shall be bound under this indemnity agreement to liability determined in a proceeding in which it did not participate represented by its own independent counsel. The indemnities provided hereunder shall not terminate upon the termination or expiration of this Agreement, but may be modified to the extent of any waiver of subrogation agreed to by TSC and paid for by Client.

12. SUBPOENAS: TSC's employees shall not be retained as expert witnesses except by separate, written agreement. Client agrees to pay TSC pursuant to TSC's then current fee schedule for any TSC employee(s) subpoenaed by any party as an occurrence witness as a result of TSC's services.

13. OTHER AGREEMENTS: TSC shall not be bound by any provision or agreement (i) requiring or providing for arbitration of disputes or controversies arising out of this Agreement or its performance, (ii) wherein TSC waives any rights to a mechanics lien or surety bond claim; (iii) that conditions TSC's right to receive payment for its services upon payment to Client by any third party or (iv) that requires TSC to indemnify any party beyond its own negligence. These General Conditions are notice, where required, that TSC shall file a lien whenever necessary to collect past due amounts. This Agreement contains the entire understanding between the parties. Unless expressly accepted by TSC in writing prior to delivery of TSC's services, Client shall not add any conditions or impose conditions which are in conflict with those contained herein, and no such additional or conflicting terms shall be binding upon TSC. The unenforceability or invalidity of any provision or provisions shall not render any other provision or provisions unenforceable or invalid. This Agreement shall be construed and enforced in accordance with the laws of the State of Illinois. In the event of a dispute arising out of or relating to the performance of this Agreement, the breach thereof or TSC's services, the parties agree to try in good faith to settle the dispute by mediation under the Construction Industry Mediation Rules of the American Arbitration Association as a condition precedent to filing any demand for arbitration, or any petition or complaint with any court. Paragraph headings are for convenience only and shall not be construed as limiting the meaning of the provisions contained in these General Conditions.

APPENDIX

UNIFIED CLASSIFICATION CHART

LEGEND FOR BORING LOGS

BORING LOGS

BORING LOCATION PLAN

Testing Service Corporation

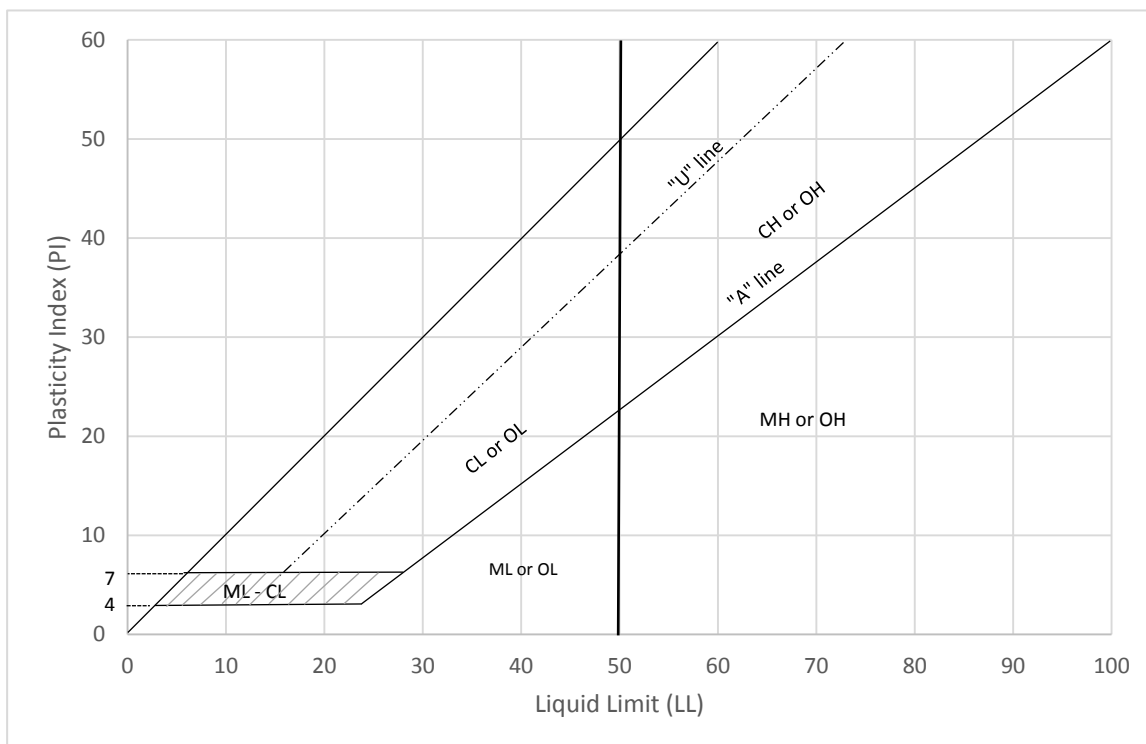
Unified Classification Chart



CRITERIA FOR ASSIGNING GROUP SYMBOLS AND GROUP NAMES USING LABORATORY TEST °				SOIL CLASSIFICATION	
				Group Symbol	GROUP NAME ^b
COARSE - GRAINED SOILS more than 50% retained on No. 200 sieve	GRAVELS More than 50% of coarse fraction retained on No. 4 sieve	CLEAN GRAVELS less than 5% fines ^c	$c_u \geq 4$ and $1 \leq c_c \leq 3$ ^e	GW	Well-graded gravel ^f
			$c_u < 4$ and/or $1 > c_c > 3$ ^e	GP	Poorly-graded gravel ^f
		GRAVELS WITH FINES more than 12% fines ^c	Fines classify as ML or MH	GM	Silty gravel ^{f, g, h}
			Fines classify as CL or CH	GC	Clayey gravel ^{f, g, h}
	SANDS 50% or more of coarse fraction passes No. 4 sieve	CLEAN SANDS less than 5% fines ^d	$c_u \geq 6$ and $1 \leq c_c \leq 3$ ^e	SW	Well-graded sand ⁱ
			$c_u < 6$ and/or $1 > c_c > 3$ ^e	SP	Poorly-graded sand ⁱ
		SANDS WITH FINES more than 12% fines ^d	Fines classify as ML or MH	SM	Silty sand ^{g, h, f}
			Fines classify as CL or CH	SC	Clayey sand ^{g, h, f}
FINE - GRAINED SOILS 50% or more passed the No. 200 sieve	SILTS & CLAYS Liquid limit less than 50%	Inorganic	PI > 7 or plots on or above "A" line j	CL	Lean clay ^{k, l, m}
			PI < 4 or plots below "A" line j	ML	Silt ^{k, l, m}
		Organic	$\frac{\text{Liquid limit} - \text{oven dried}}{\text{Liquid limit} - \text{not dried}} < 0.75$	OL	Organic clay ^{k, l, m, n} Organic silt ^{k, l, m, o}
	SILTS & CLAYS Liquid limit 50% or more	Inorganic	PI plots on or above "A" line	CH	Fat clay ^{k, l, m}
			PI plots below "A" line	MH	Elastic silt ^{k, l, m}
		Organic	$\frac{\text{Liquid limit} - \text{oven dried}}{\text{Liquid limit} - \text{not dried}} < 0.75$	OH	Organic clay ^{k, l, m, p} Organic silt ^{k, l, m, q}
	Highly organic soils		Primarily organic matter, dark in color, and organic odor		PT

- a. Based on the material passing the 3-inch (75-mm) sieve.
b. If field sample contained cobbles and/or boulders, add "with cobbles and/or boulders" to group name
c. Gravels with 5 to 12% fines require dual symbols
GW-GM well graded gravel with silt
GW-GC well graded gravel with clay
GP-GM poorly graded gravel with silt
GP-GC poorly graded gravel with clay
d. Sands with 5 to 12% fines require dual symbols
SW-SM well graded sand with silt
SW-SC well graded sand with clay
SP-SM poorly graded sand with silt
SP-SC poorly graded sand with clay
e. $C_u = D_{60}/D_{10}$ $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$

- f. If soils contains $\geq 15\%$ sand, add "with sand" to group name.
g. If fines classify as CL-ML, use dual symbol GC-GM, SC-SM
h. If fines are organic, add "with organic fines" to group name
i. If soils contains $\geq 15\%$ gravel, add "with gravel" to group name
j. If Atterberg Limits plot in hatched area, soil is a CL – ML, silty clay
k. If soils contains 15 to 29% plus No. 200, add "with sand" or "with gravel" whichever is predominant
l. If soil contains $\geq 30\%$ plus No. 200, predominantly sand, add "sandy" to group name.
m. If soils contains $\geq 30\%$ plus No. 200, predominantly gravel, add "gravelly" to group name
n. PI ≥ 4 and plots on or above "A" line
o. PI ≥ 4 and plots below "A" line
p. PI plots on or above "A" line
q. PI plots below "A" line



LEGEND FOR BORING LOGS



FILL



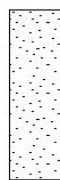
TOPSOIL



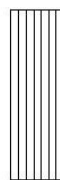
PEAT



GRAVEL



SAND



SILT



CLAY



LIMESTONE/
DOLOMITE

SAMPLE TYPE

SS	=	Split-Spoon
ST	=	Thin-Walled Tube
A	=	Auger
MC	=	Macro-Core (Geoprobe)

WATER LEVEL OBSERVATIONS

▼	While Drilling
▽	End of Boring
▼	24 Hours

FIELD AND LABORATORY TEST DATA

N	=	Standard Penetration Resistance in Blows per Foot (bpf)
WC	=	In-Situ Water Content (%)
Qu	=	Unconfined Compressive Strength in Tons per Square Foot (tsf)
*	=	Pocket Penetrometer Reading: Maximum Value = 4.5 tsf
γ _{dry}	=	Dry Unit Weight in Pounds per Cubic Foot (pcf)

SOIL DESCRIPTIONS:

MATERIAL

BOULDER
COBBLE
Large GRAVEL
Small GRAVEL
Coarse SAND
Medium SAND
Fine SAND
SILT and CLAY

PARTICLE SIZE RANGE

Over 12 inches
12 inches to 3 inches
3 inches to ¾ inch
¾ inch to No. 4 Sieve
No. 4 Sieve to No. 10 Sieve
No. 10 Sieve to No. 40 Sieve
No. 40 Sieve to No. 200 Sieve
Passing No. 200 Sieve

COHESIVE SOILS

<u>CONSISTENCY</u>	<u>Qu (tsf)</u>
Very Soft	Less than 0.25
Soft	0.25 to 0.5
Medium Stiff	0.5 to 1.0
Stiff	1.0 to 2.0
Very Stiff	2.0 to 4.0
Hard	4.0 and over

COHESIONLESS SOILS

<u>RELATIVE DENSITY</u>	<u>N (bpf)</u>
Very Loose	0 – 3
Loose	4 – 9
Medium Dense	10 – 29
Dense	30 – 49
Very Dense	50 and over

MODIFYING TERM

Trace
Little
Some

PERCENT BY WEIGHT

1 – 10
10 – 20
20 – 35

TESTING SERVICE CORPORATION457 East Gundersen Drive
Carol Stream, IllinoisTSC Job No. L-101,160
Page 1 of 1**Client:** Lisle Library District
777 Front Street
Lisle, IL 60532**Project:** Lisle Library Improvements
Gateway Sign, Pedestrian Bridge & Nature Trail
777 Front Street
Lisle, Illinois**SOIL TEST DATA**

BORING NUMBER	1	2	2	
SAMPLE NUMBER	Combined 2 &3	3	4	
DEPTH IN FEET	3.5 - 7.5	6.0 - 7.5	8.5 - 10.0	
UNIFIED SOIL CLASSIFICATION	CL	CL	CL	
GRADATION - PASSING 1½" SIEVE %	100	100	100	
GRADATION - PASSING 1" SIEVE %	100	100	100	
GRADATION - PASSING ¾" SIEVE %	100	100	100	
GRADATION - PASSING 3/8" SIEVE %	98.6	100	87.2	
GRADATION - PASSING # 4 SIEVE %	97.8	97.4	67.9	
GRADATION - PASSING # 10 SIEVE %	91.6	93.4	59.5	
GRADATION - PASSING # 40 SIEVE %	86.1	88.4	54.8	
GRADATION - PASSING # 100 SIEVE %	81.1	83.2	50.0	
GRADATION - PASSING # 200 SIEVE %	78.0	80.3	47.9	
USDA PARTICLE SIZE RANGES				
SAND % (0.05 mm - 2.0 mm)	17.0	15.8	24.2	
SILT % (0.002 mm - 0.05mm)	53.0	49.7	47.4	
CLAY % (finer than 0.002 mm)	21.6	34.5	28.4	
USDA SOIL TEXTURE CLASSIFICATION	---	Silty Clay Loam	Very Gravelly Clay Loam	
ESTIMATED INFILTRATION (IN/HR)	---	0.19	0.03	
LIQUID LIMIT %	31	---	---	
PLASTIC LIMIT %	21	---	---	
PLASTICITY INDEX %	10	---	---	

PROJECT **LD Nature Trail CD-CA, 777 Front Street, Lisle, IL**CLIENT **Lisle Library District, 777 Front Street, Lisle, IL**STARTED **08/13/2026** FINISHED **08/13/2026** DRILL RIG NO. **314**JOB **L-101,160**

BORING				ELEVATIONS							WATER LEVEL OBSERVATIONS					
1				GROUND SURFACE		718.0							▽ WHILE DRILLING		1.0'	
				END OF BORING		688.0							▽ AT END OF BORING		1.0'	
Depth (ft.)	Lithology	Length	Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS			
				NO.	TYPE											
0											0.4	717.6	▽ 5" Bituminous Concrete			
				1	SS	11	15.3	4.0			1.3	716.7	▽ 8" Crushed Stone			
													Very stiff to hard brown silty CLAY, little to some sand and gravel, moist (CL)			
				2	SS	12	17.4	0.75			3.0	715.0	Medium stiff brown silty CLAY, little to some sand, trace gravel, very moist (CL)			
5				3	SS	18	20.2	0.75					Combined Samples 2 & 3: LL=31, PL=21, PI=10 Gravel 2%, Sand 20%, Silt 42%, Clay 36%			
				4	SS	20	14.8	4.5+			8.0	710.0	Hard brown and gray silty CLAY, little to some sand and gravel, moist (CL)			
10				5	SS	20	16.6	4.25	3.93		10.5	707.5	Hard gray silty CLAY, little to some sand and gravel, moist (CL)			
				6	SS	16	22.6	2.0	1.89		13.0	705.0	Stiff to very stiff gray silty CLAY, little sand, trace gravel, moist (CL)			
15				7	SS	11	14.4	1.25			15.5	702.5	Stiff gray silty CLAY, some sand, little gravel, moist to very moist (CL)			
				8	SS	14	12.7	3.25			18.0	700.0	Very stiff gray silty CLAY, little to some sand and gravel, moist (CL)			
20				9	SS	12	16.5	2.25					Very stiff gray silty CLAY, little to some sand and gravel, moist (CL)			
				10	SS	14	17.3	2.5			25.5	692.5	Very stiff gray silty CLAY, some sand and gravel, occasional Cobbles and Boulders, moist (CL)			
25				11	SS	88	13.3	3.5			28.0	690.0	Stiff to very stiff gray silty CLAY, little sand, trace gravel, occasional sand seams, moist (CL)			
				12	SS	21	19.6	2.0					End of Boring at 30.0'			
30																
35																

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **LD Nature Trail CD-CA, 777 Front Street, Lisle, IL**CLIENT **Lisle Library District, 777 Front Street, Lisle, IL**STARTED **08/13/2026** FINISHED **08/13/2026** DRILL RIG NO. **314**JOB **L-101,160**

BORING			ELEVATIONS								WATER LEVEL OBSERVATIONS					
2			GROUND SURFACE		722.5								▽ WHILE DRILLING		20.0'	
			END OF BORING		692.5								▽ AT END OF BORING		23.0'	
Depth (ft.)	Lithology	Length	Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS			
				NO.	TYPE											
0											0.8	721.7	Black clayey TOPSOIL (OL)			
		1	SS	8	24.8	3.0					3.0	719.5	Very stiff brown silty CLAY, trace sand, moist (CL/CH)			
5		2	SS	13	15.4	4.5+							Very stiff to hard brown silty CLAY, some sand, little gravel, moist (CL)			
		3	SS	20	17.0	3.75										
		4	SS	27	13.5	3.25										
		5	SS	28	14.4	4.5+	4.66				13.0	709.5				
10		6	SS	16	14.8	3.0	2.93				15.5	707.0	Very stiff brown and gray silty CLAY, little to some sand and gravel, moist (CL)			
15		7	SS	15	16.6	3.75					18.0	704.5	Stiff to very stiff gray silty CLAY, little sand and gravel, moist (CL)			
20		8	SS	20	13.6	2.0					20.5	702.0	▽	Stiff to very stiff gray sandy CLAY, trace gravel, moist (CL)		
25		9	SS	13	15.2						23.0	699.5	▽	Medium dense gray silty SAND, wet (SM)		
30		10	SS	12	15.0	1.5							Stiff gray silty CLAY, some sand, trace gravel, occasional sand seams, moist to very moist (CL)			
		11	SS	11	15.2	1.5					28.0	694.5				
	12	SS	13	16.8	2.25							Very stiff gray silty CLAY, little sand and gravel, moist (CL)				
35													End of Boring at 30.0'			

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **LD Nature Trail CD-CA, 777 Front Street, Lisle, IL**CLIENT **Lisle Library District, 777 Front Street, Lisle, IL**STARTED **08/13/2026** FINISHED **08/13/2026** DRILL RIG NO. **314**JOB **L-101,160**

BORING	ELEVATIONS	WATER LEVEL OBSERVATIONS
3	GROUND SURFACE 722.5	▼ WHILE DRILLING Dry
	END OF BORING 712.5	▽ AT END OF BORING Dry

Depth (ft.)	Lithology	Length Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
			NO.	TYPE								
0			1A	SS	6	28.6	1.0			1.5	721.0	Black clayey TOPSOIL, very moist (OL)
			1B	SS		27.1	2.0			3.0	719.5	Stiff to very stiff dark brown silty CLAY, trace sand, trace organic, moist (CL/CH)
5			2	SS	13	16.5	3.0					Very stiff to hard brown silty CLAY, little sand and gravel, moist (CL)
			3	SS	18	17.7	4.5					
			4	SS	21	17.2	4.0					
10												End of Boring at 10.0'
15												
20												
25												
30												
35												

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **LD Nature Trail CD-CA, 777 Front Street, Lisle, IL**CLIENT **Lisle Library District, 777 Front Street, Lisle, IL**STARTED **08/13/2026** FINISHED **08/13/2026** DRILL RIG NO. **314**JOB **L-101,160**

BORING	ELEVATIONS	WATER LEVEL OBSERVATIONS
4	GROUND SURFACE 724.0	WHILE DRILLING Dry
	END OF BORING 714.0	AT END OF BORING Dry

Depth (ft.)	Lithology	Length Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
			NO.	TYPE								
0			1A	SS	6	32.8	0.75			1.5	722.5	Black clayey TOPSOIL, very moist (OL)
			1B	SS		21.4	3.25			3.0	721.0	Very stiff brown silty CLAY, little sand, moist (CL)
5			2	SS	12	16.2	3.0					Very stiff to hard brown silty CLAY, little to some sand and gravel, moist (CL)
			3	SS	17	14.2	4.0					
			4	SS	23	14.8	4.5					
10												End of Boring at 10.0'
15												
20												
25												
30												
35												

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

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BORING	ELEVATIONS	WATER LEVEL OBSERVATIONS
5	GROUND SURFACE 725.0	▼ WHILE DRILLING Dry
	END OF BORING 715.0	▽ AT END OF BORING Dry

Depth (ft.)	Lithology	Length Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
			NO.	TYPE								
0										0.8	724.2	Black clayey TOPSOIL (OL)
			1	SS	6	28.0	2.75			3.0	722.0	Very stiff dark brown silty CLAY, trace sand, trace organic, moist (CL/CH)
5			2	SS	20	17.1	4.5					Hard to very stiff brown silty CLAY, little to some sand and gravel, moist (CL)
			3	SS	17	15.0	4.5					
			4	SS	27	17.4	2.5					
10												End of Boring at 10.0'
15												
20												
25												
30												
35												

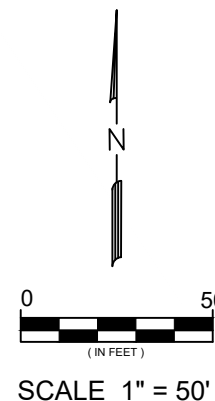
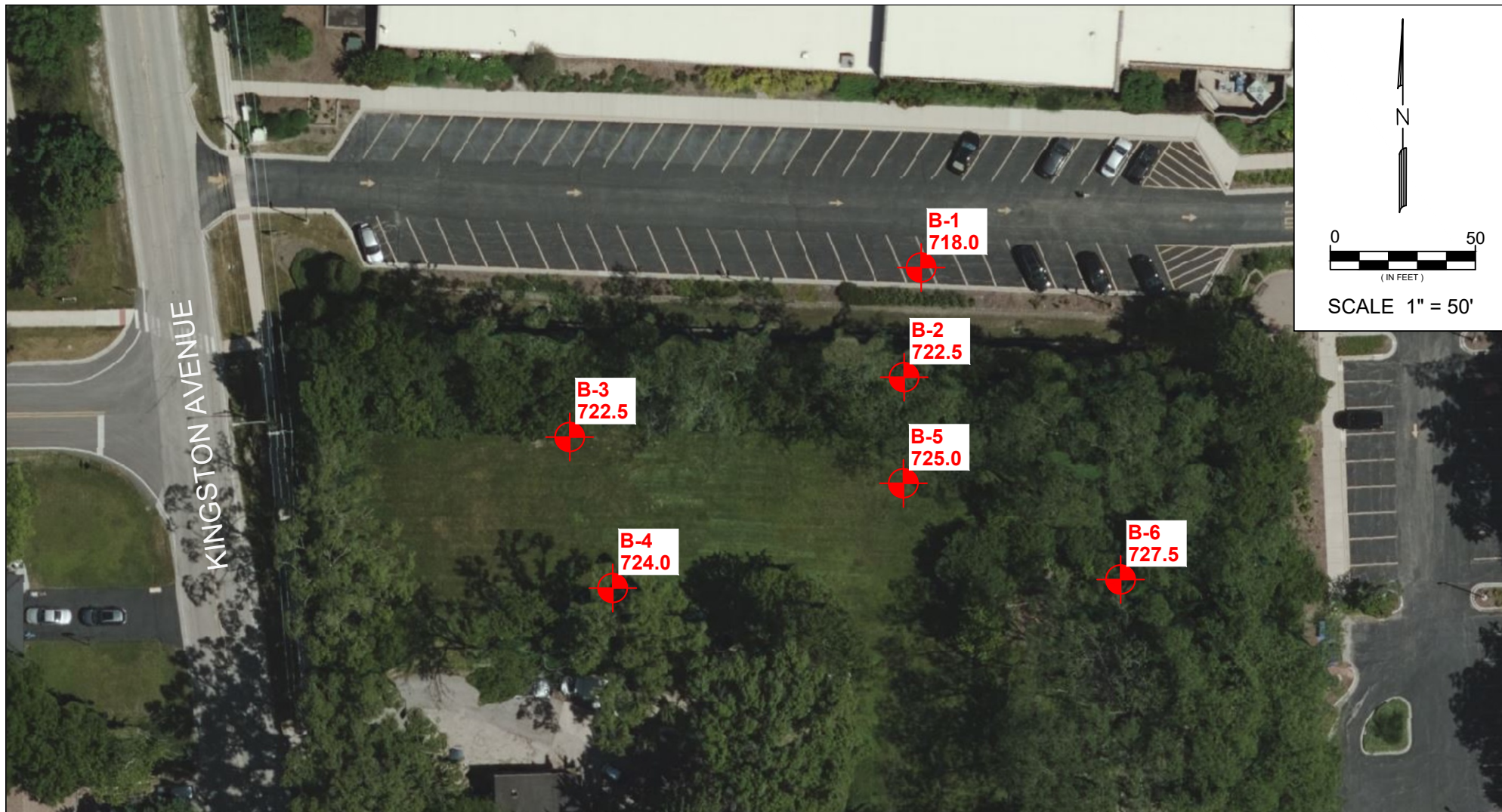
Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **LD Nature Trail CD-CA, 777 Front Street, Lisle, IL**CLIENT **Lisle Library District, 777 Front Street, Lisle, IL**STARTED **08/13/2026** FINISHED **08/13/2026** DRILL RIG NO. **314**JOB **L-101,160**

BORING	ELEVATIONS	WATER LEVEL OBSERVATIONS
6	GROUND SURFACE 727.5	WHILE DRILLING Dry
	END OF BORING 717.5	AT END OF BORING Dry

Depth (ft.)	Lithology	Length Recovery	SAMPLE		N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
			NO.	TYPE								
0			1A	SS	7	32.6	3.0			1.5	726.0	Black clayey TOPSOIL (OL)
			1B	SS	7	23.1	4.0			3.0	724.5	Very stiff to hard dark brown silty CLAY, little sand, trace organic, moist (CL)
5			2	SS	14	14.2	4.0					Very stiff to hard brown silty CLAY, little to some sand and gravel, moist (CL)
			3	SS	19	15.9	4.5+			8.0	719.5	Very stiff brown silty CLAY, little sand and gravel, moist (CL)
			4	SS	10	18.7	3.25					Very stiff brown silty CLAY, little sand and gravel, moist (CL)
10												End of Boring at 10.0'
15												
20												
25												
30												
35												

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.



NOTE: GROUND SURFACE ELEVATIONS AT THE BORINGS WERE ACQUIRED BY TSC USING A TRIMBLE R12 GNSS RECEIVER, BEING ROUNDED TO THE NEAREST 0.5 FOOT.

LEGEND



BORING LOCATION PLAN
LD NATURE TRAIL CD-CA
777 FRONT STREET
LISLE, ILLINOIS



TESTING SERVICE CORP.
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

DRAWN BY: FFE

CHECKED BY: SJP

JOB NO.: L-101,160

DATE: 08-06-26

PAGE NO.

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